

# COST Action FP1004

## Final Meeting

15 April – 17 April 2015 – Lisbon, Portugal



## Stress-laminated-timber decks – state of the art and design based on Swedish practice

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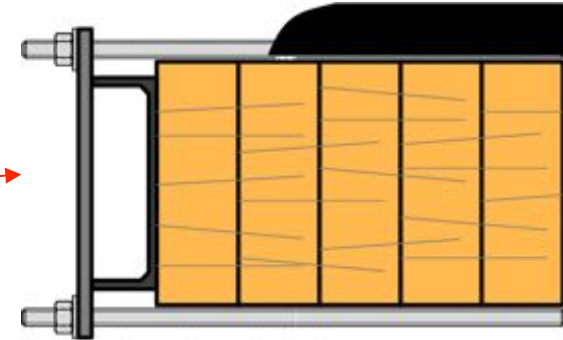


- Evolution of stress laminated decks
- Stress laminated timber decks
- Butt joint
- Erection
- Loss of pre-stress
- Design models
- Details and weather protection
- Durability
- Conclusions

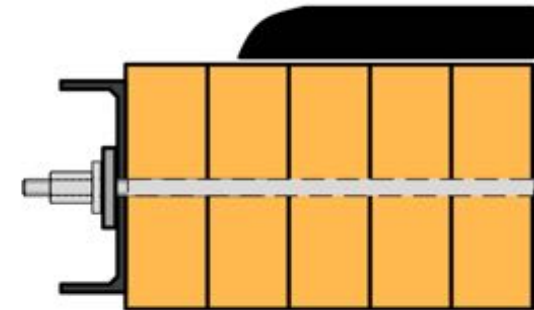
# Evolution of stress laminated timber bridges



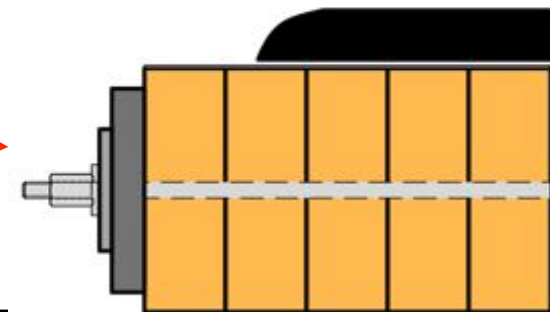
❖ Nail-laminated timber bridges (Unserviceable due to gaps)



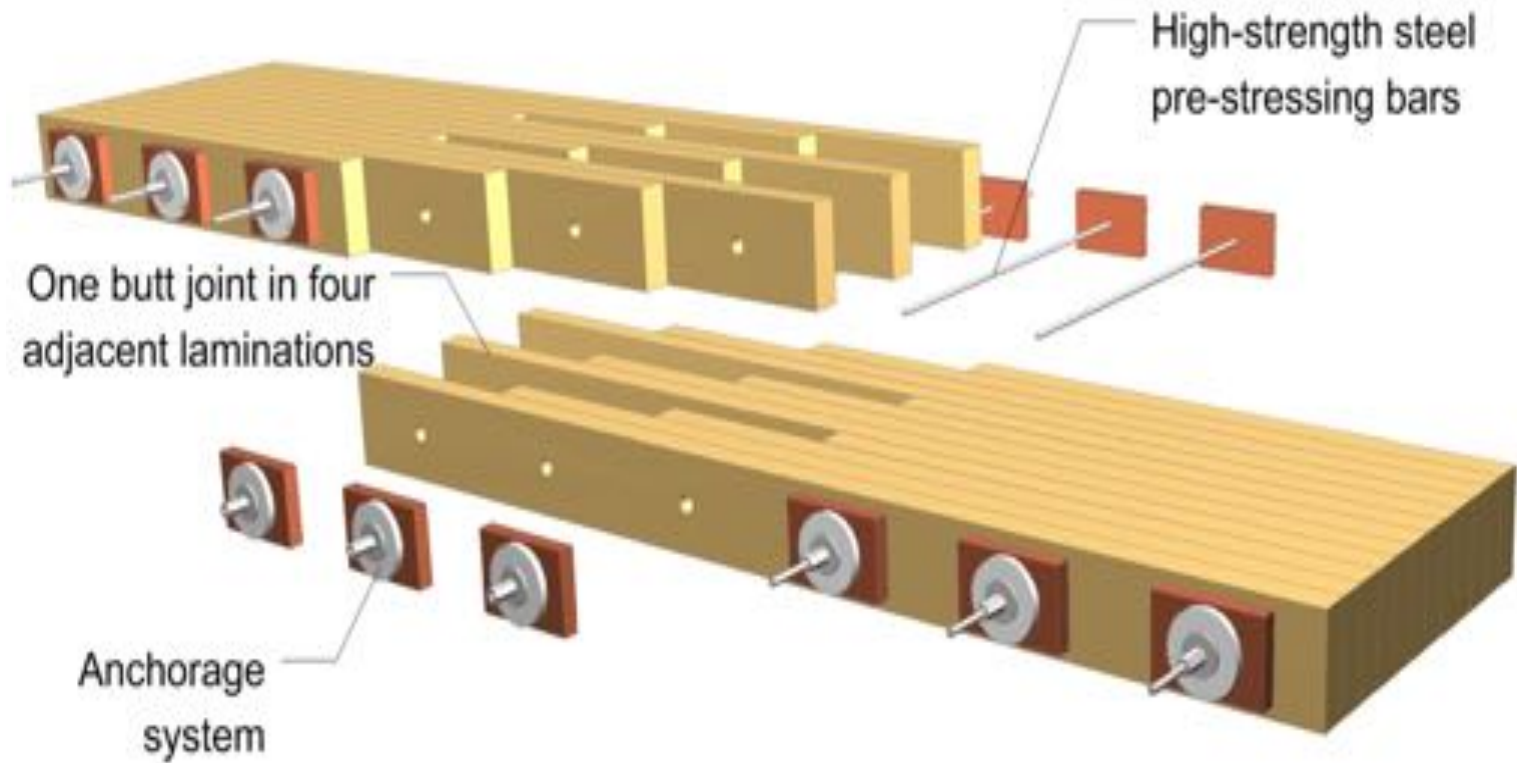
❖ Continuous steel profiles



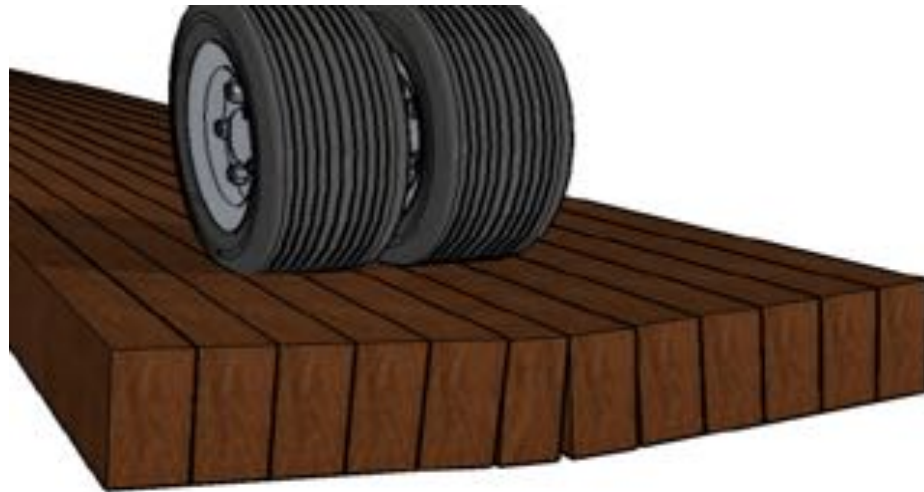
❖ Patch anchoring with plates



# Stress laminated timber decks



# Why do we need to prestress?



Transverse bending



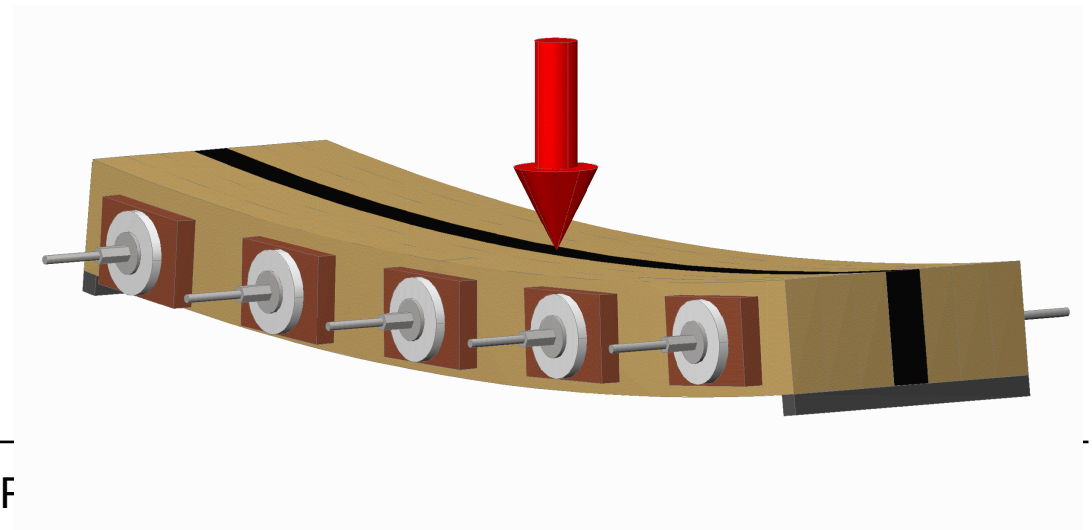
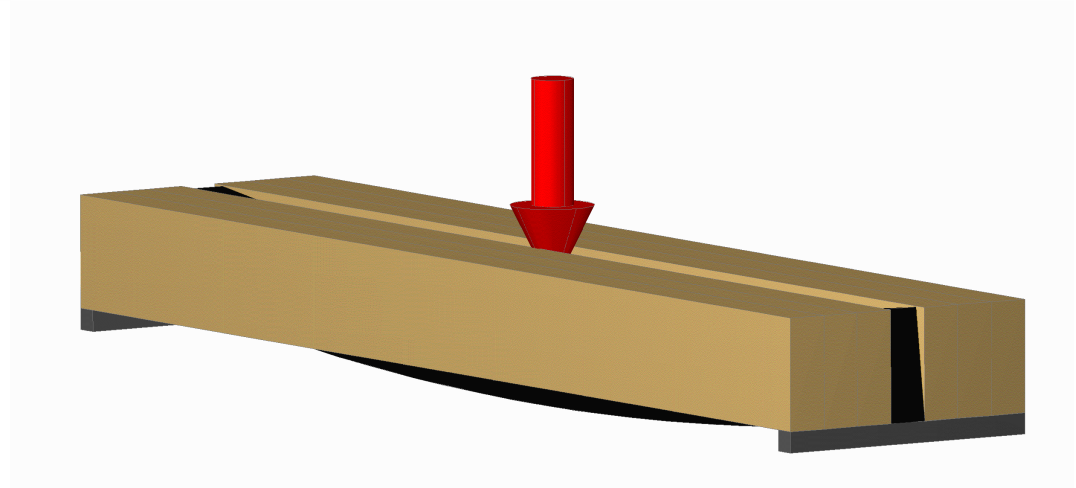
Transverse shear

# Stress laminated timber deck - principles



## Load distribution through friction

- ❖ Compressive stresses between laminations
- ❖ Redistribution of concentrated loads





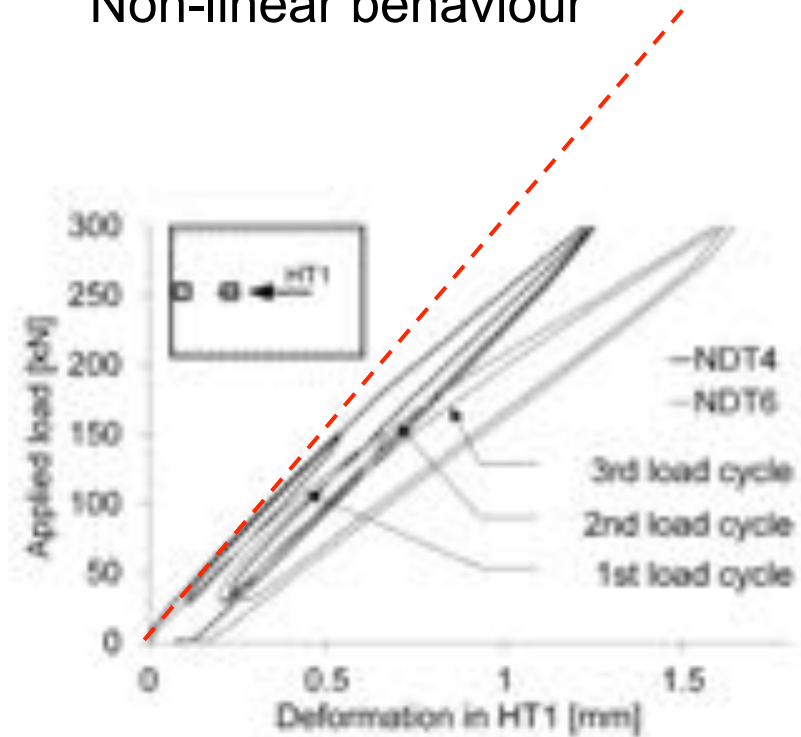
# Other “movements” in the deck



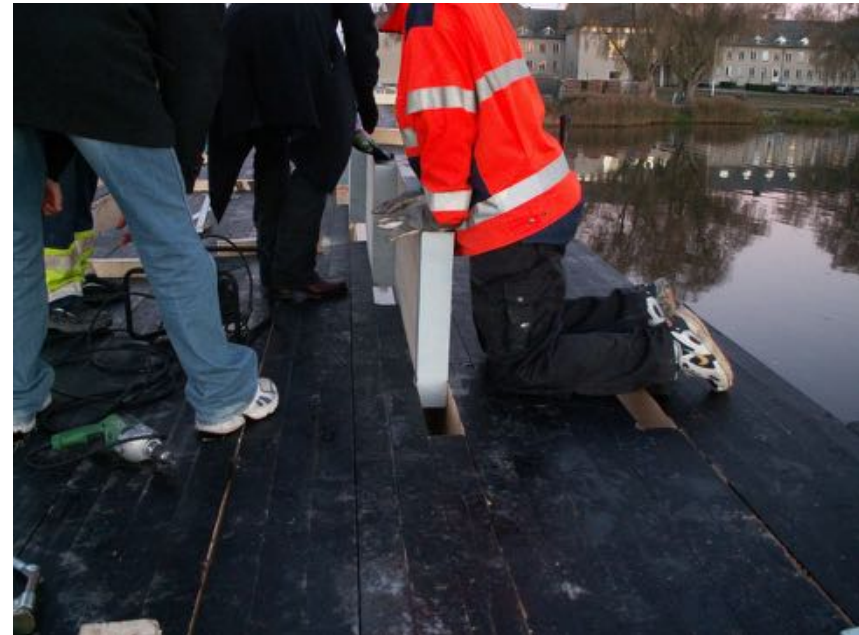
Twist due to torsion in the deck



Non-linear behaviour



# Butt Joints

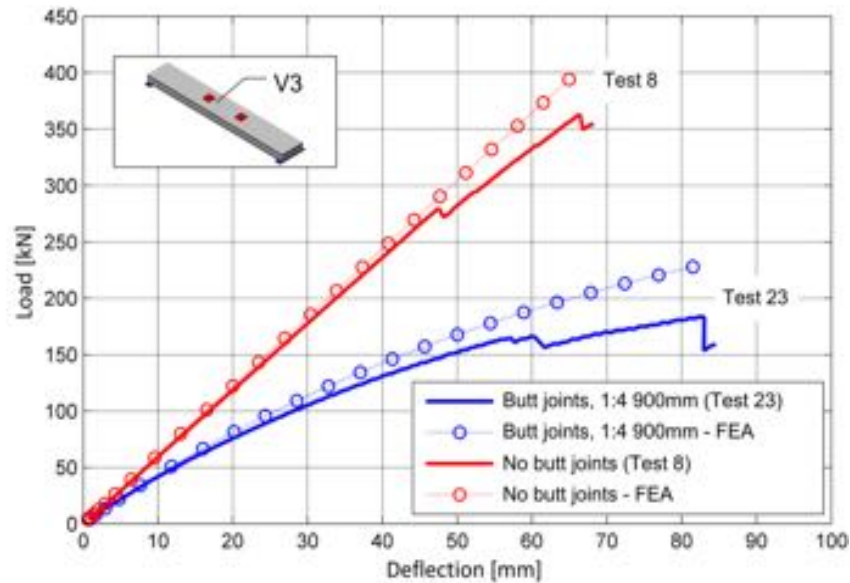




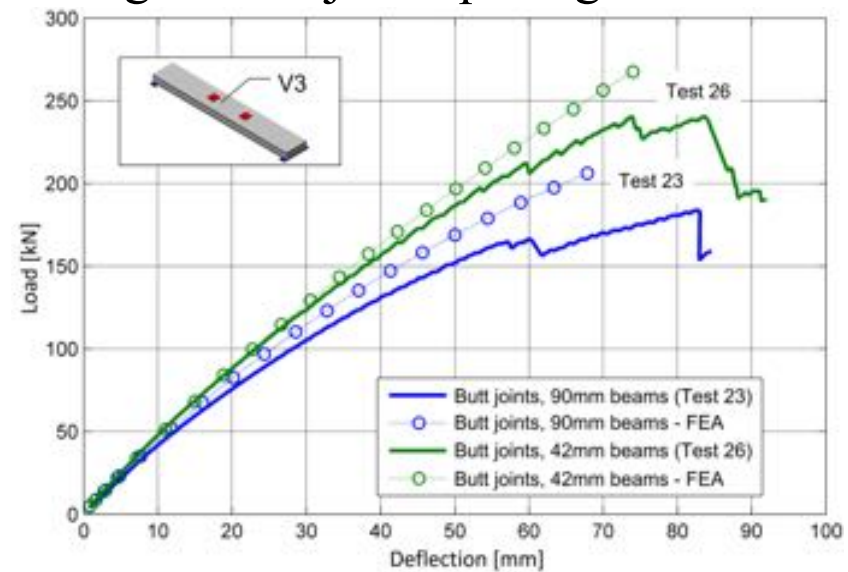
# Butt Joints



Comparison of specimens with and without butt joints



Comparison of 42 mm and 90 mm beam width for specimens with one-in-four spaced beams butt jointed with a longitudinal joint spacing of 900 mm



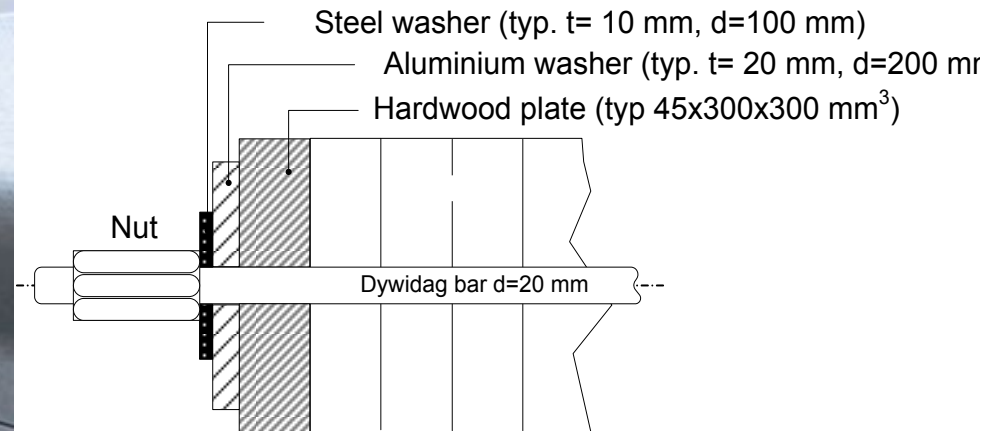
# Erection



**Pedestrian bridge,  $t=495\text{mm}$ ,  
 $B = 3\text{m}$ ,  $L_{\text{tot}} \approx 56\text{m}$ ,  $s_{\text{max}} \approx 17\text{m}$**



# Erection



Initial compression stress between laminations:  $\sigma_p \approx 0,8-1,0 \text{ MPa}$

Final compression stress between laminations:  $\sigma_{p,t=\infty} \approx 0,4 \cdot \sigma_p$

# Erection





# Loss of pre-stress



Building year: 1997

Inspection year: 2004



**Lusbäckenbron, Borlänge –  
Stress-laminated box-beam Bridge**

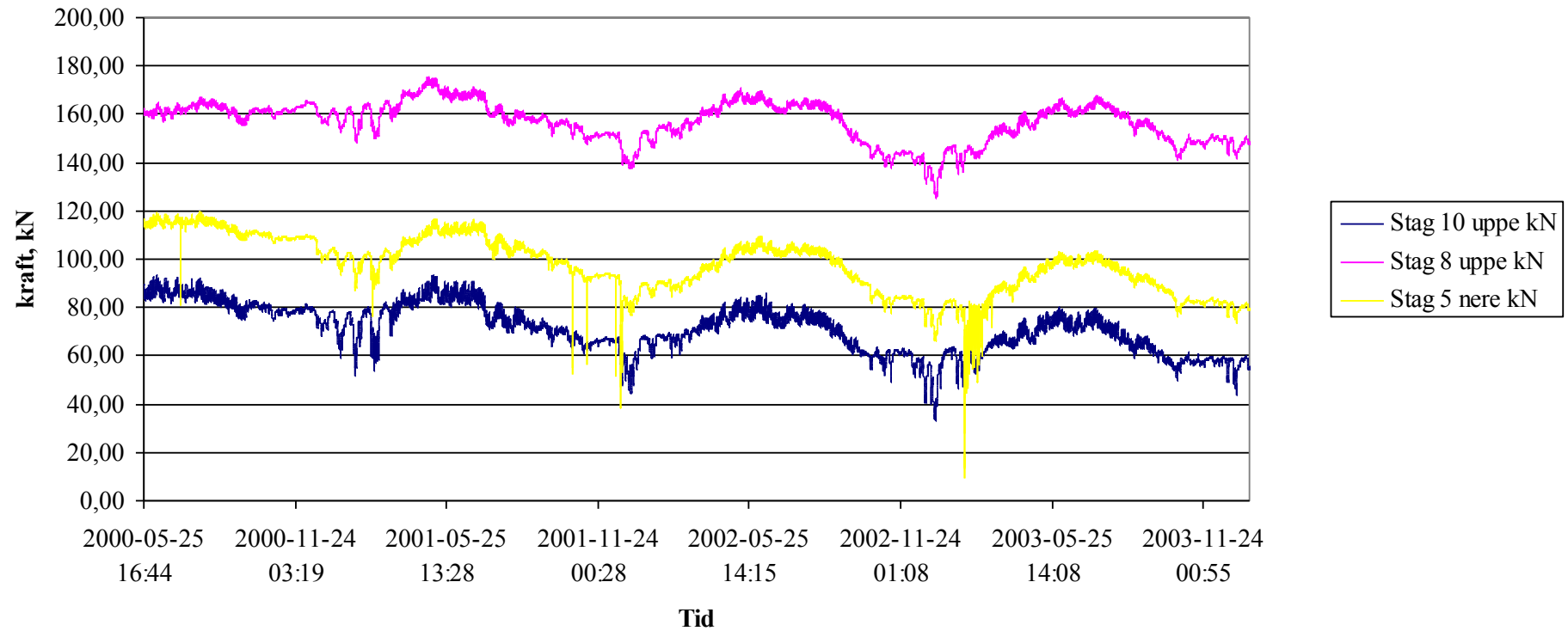
*Per-Anders Fjällström, SP-Trätekn*



# Loss of pre-stress



Stångkraft Borlänge 0005-0401



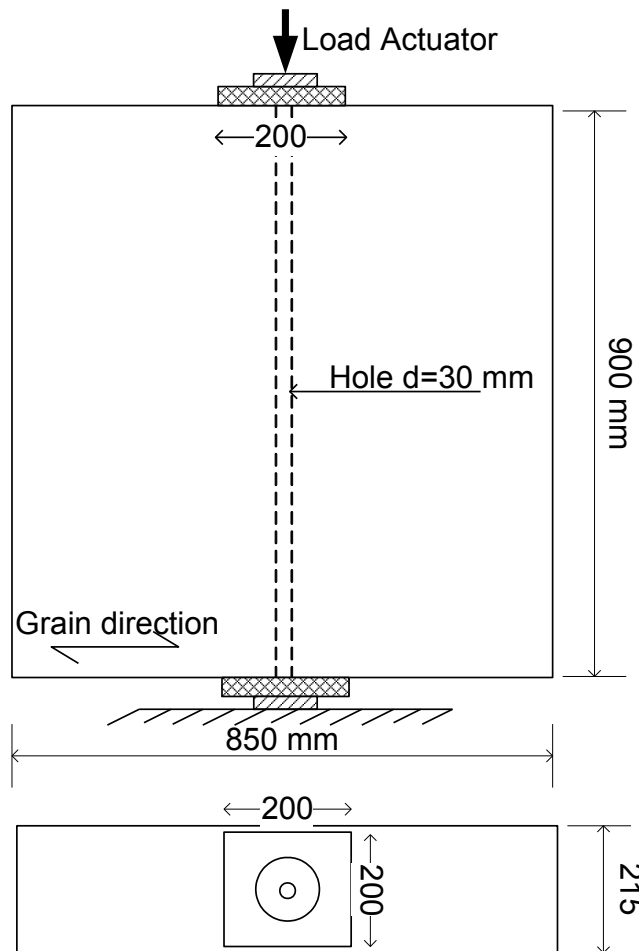
Lusbäckenbron, Borlänge –  
Stress-laminated box-beam Bridge

*after Per-Anders Fjällström, SP-Träteknik*

# Loss of pre-stress



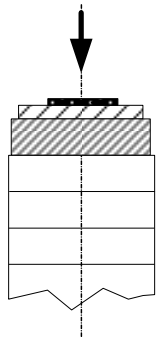
## Test series “a”: Ultimate Load Carrying Capacity



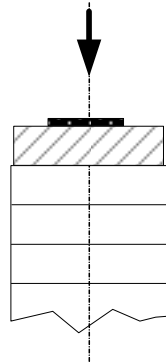
# Loss of pre-stress



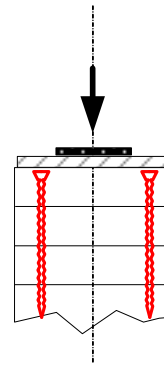
## Test series “a”: Totally 6 specimens



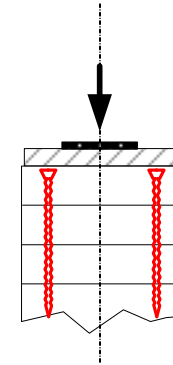
Unreinforced 1  
(plHW)



Unreinforced 2  
(pl40)



Reinforced 1 & 2  
(S10pl15)  
(S12pl15)



Reinforced 3 & 4  
(S10pl20)  
(S12pl20)

- pl: Bearing plate

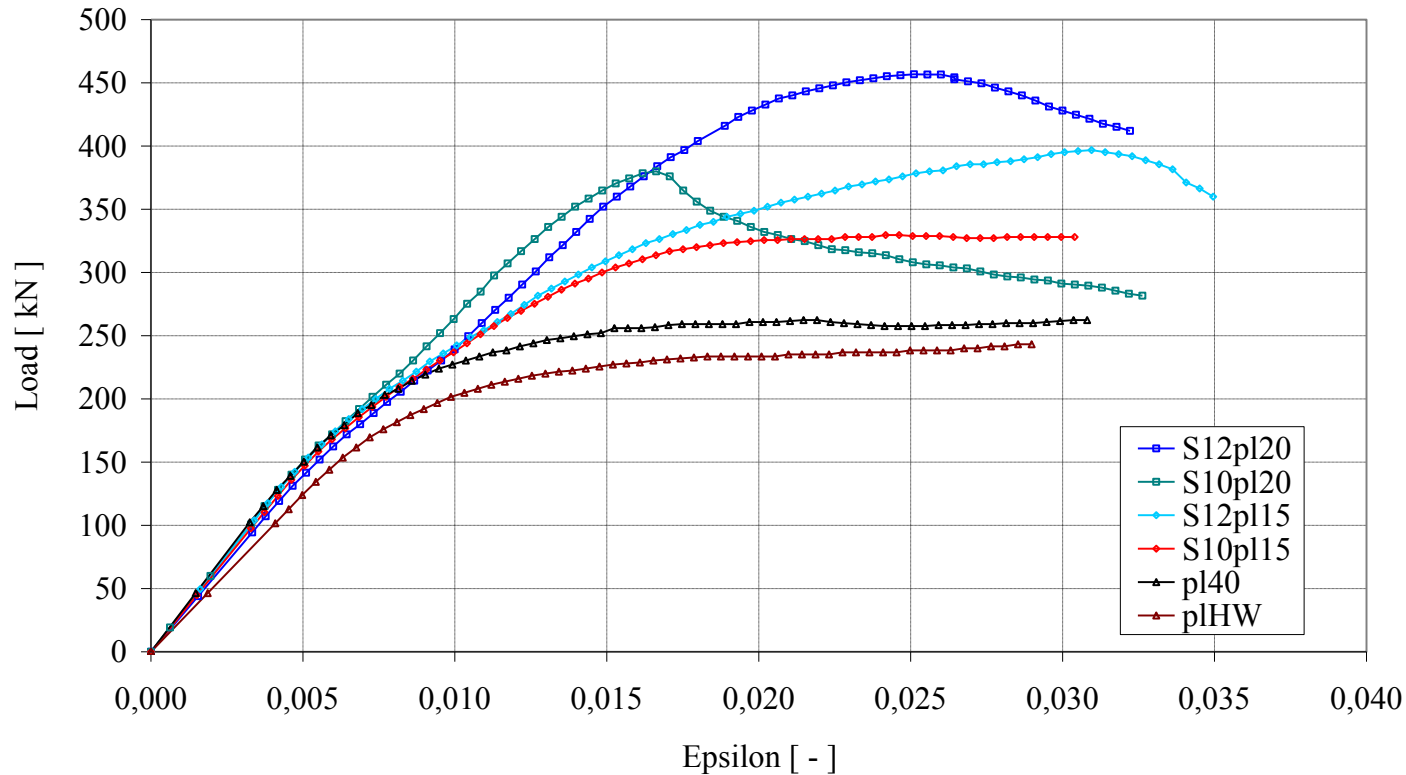
- HW: Hard Wood

- S: Screw



# Loss of pre-stress

## Test series "a": Results



The screws increase the capacity of the anchorage system by:

- ~50% if  $d=10$  mm;

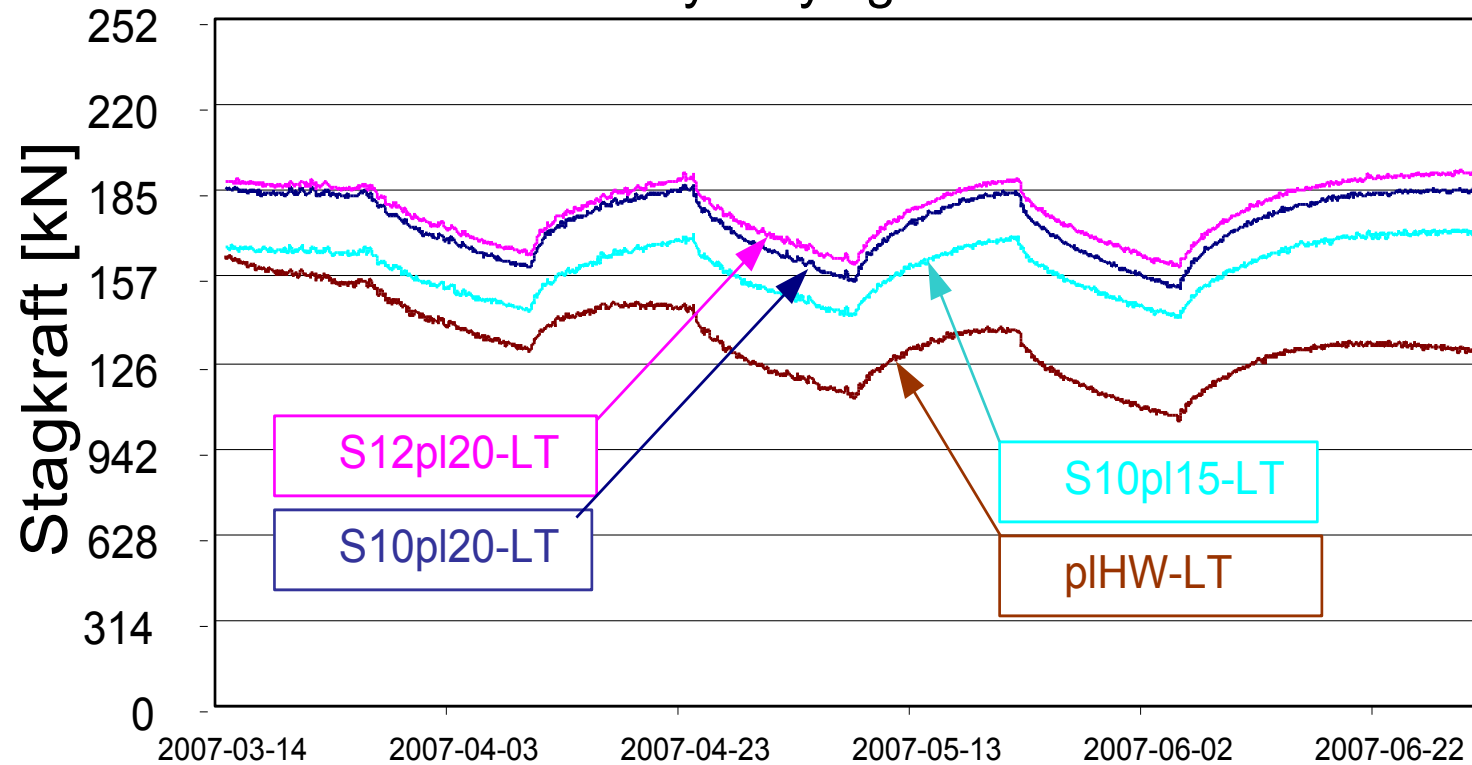
~85% if  $d=12$  mm.

# Loss of pre-stress



## Long term behaviour

Relative humidity varying between 30% and 85%



approx. 3 months

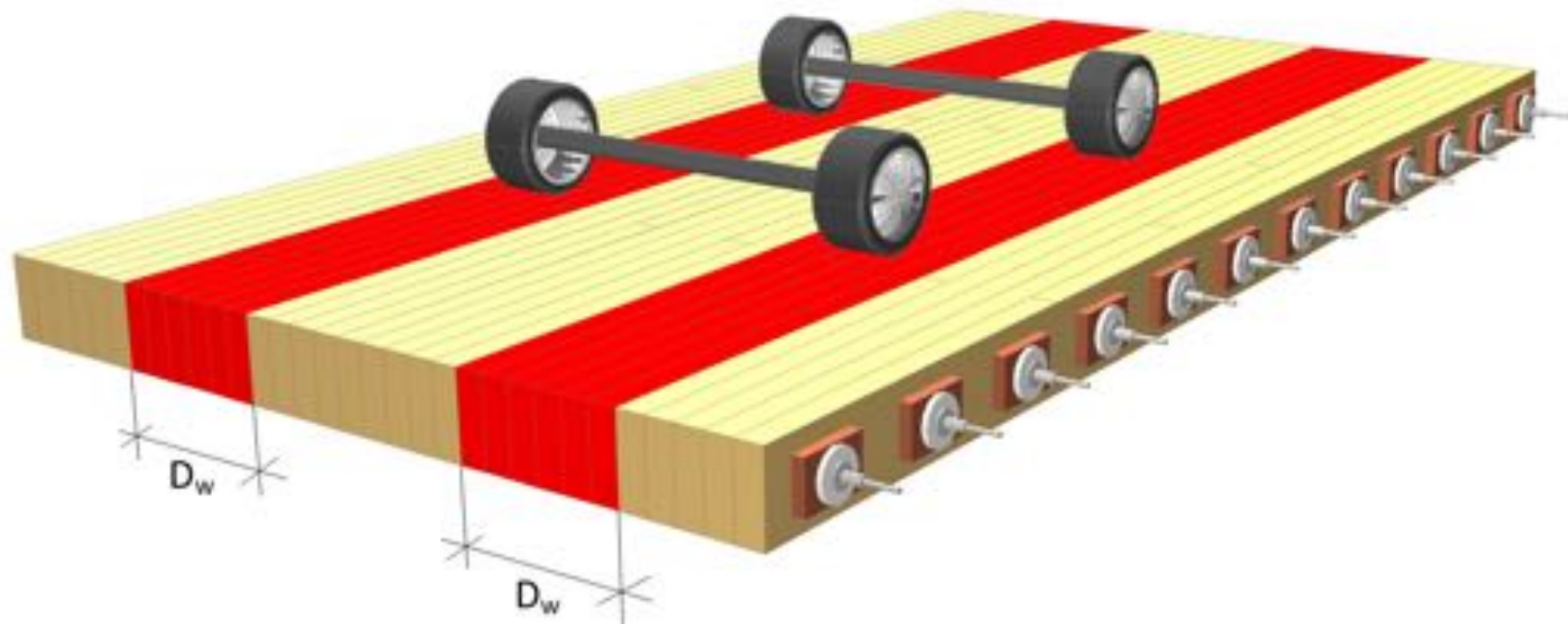


# Design models



## *Equivalent beam theory*

- Plate is substituted with beams in the wheel path, e.g. Ritter (1990), Crews (2001), EN 1995-2 (2004)



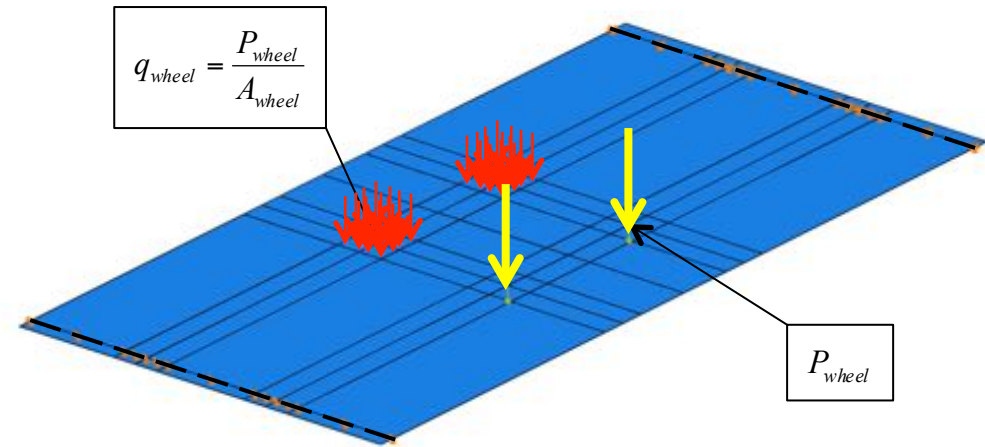
# Design models



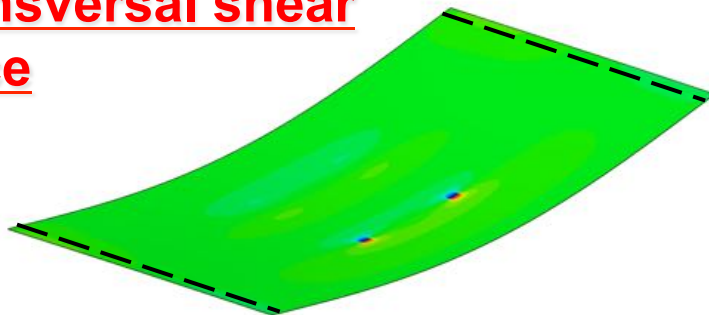
*Orthotropic plate theory by means of the finite element method*

- ❖ Patch-loads instead of concentrated loads

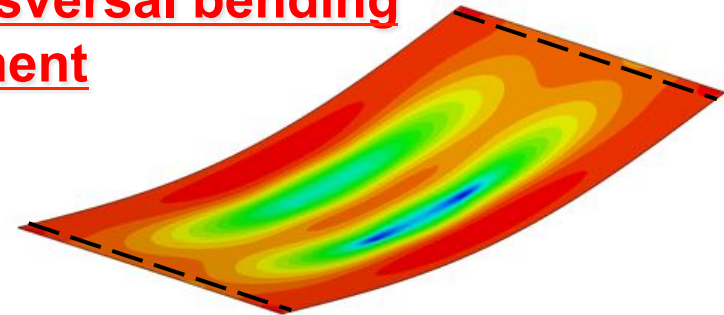
## Load and support configuration



## Transversal shear force



## Transversal bending moment



# Design models

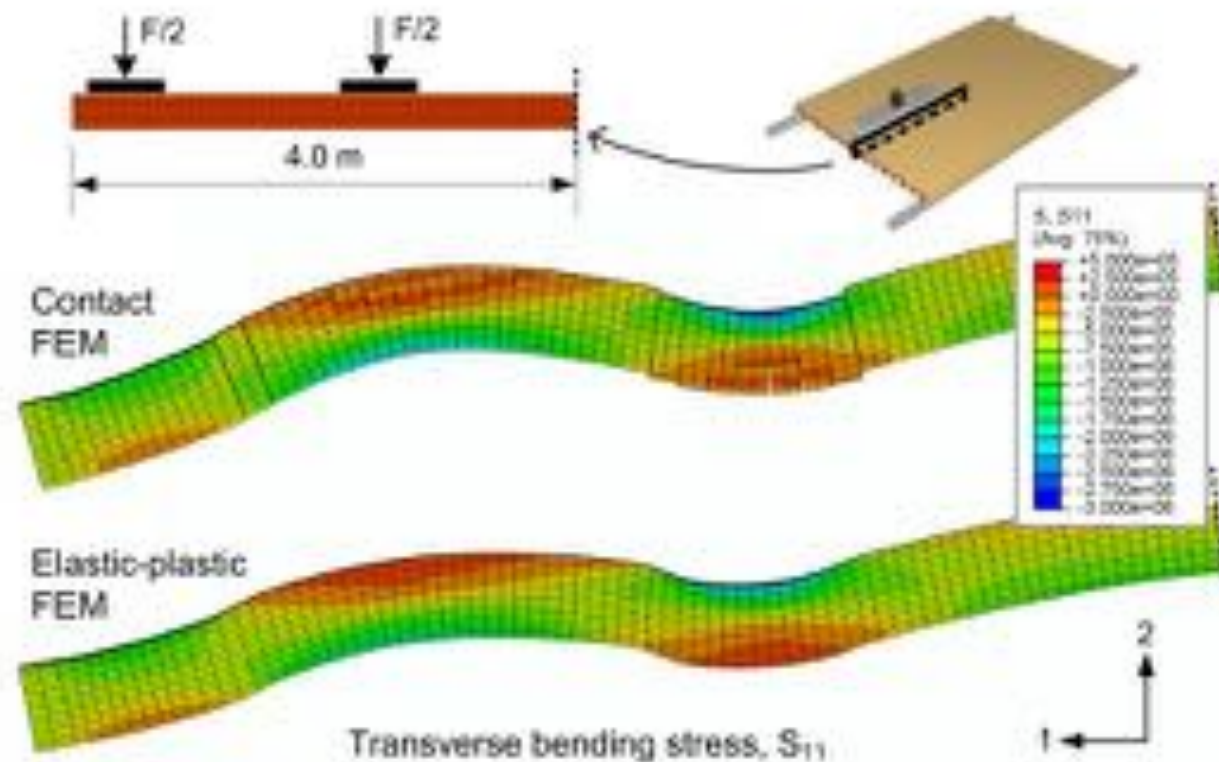


Deck type	$E_{90,mean} / E_{0,mean}$	$G_{0,mean} / E_{0,mean}$	$G_{90,mean} / G_{0,mean}$
Nail laminated	0	0.06	0.05
Stress laminated (sawn)	0.015	0.06	0.08
Stress laminated (planed)	0.02	0.06	0.10
Glue laminated	0.03	0.06	0.15

# Design models



## *Modelling by means of solid finite elements*



# Railing system



Railing with balusters of steel and a top rail of wood



Railing with both balusters and top rail made of wood





# Expansion joints

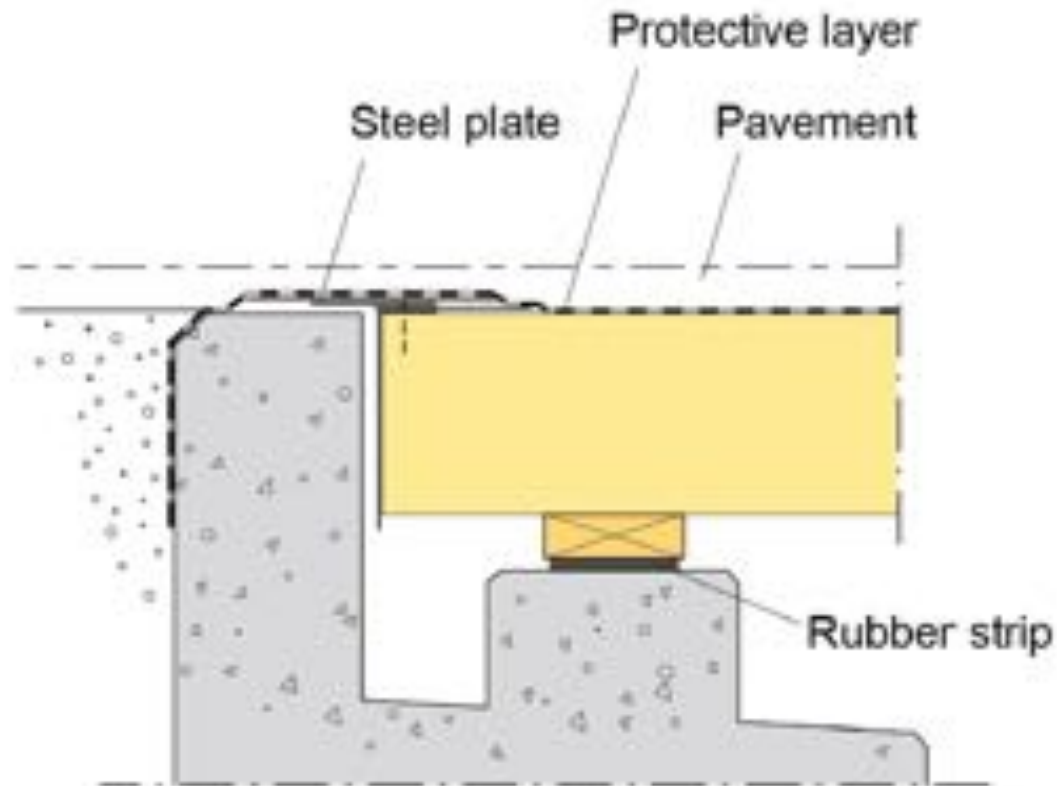
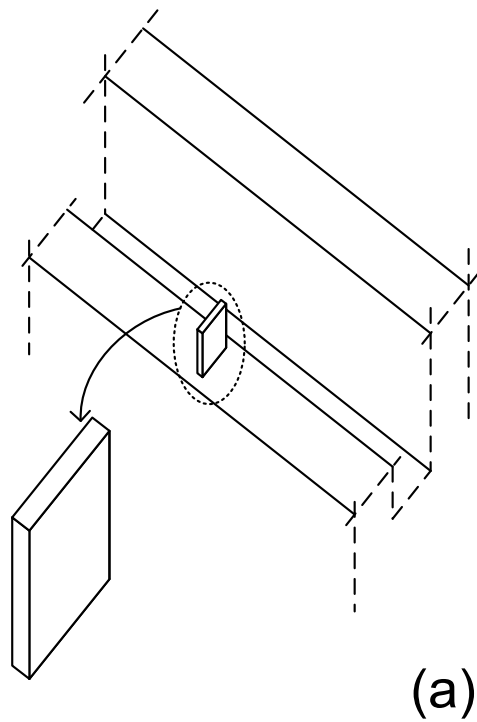
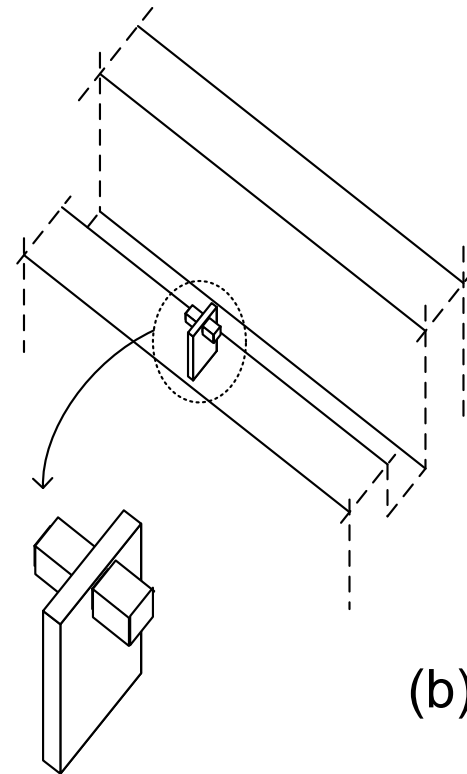


Figure: A. Pousette

# Bearing system

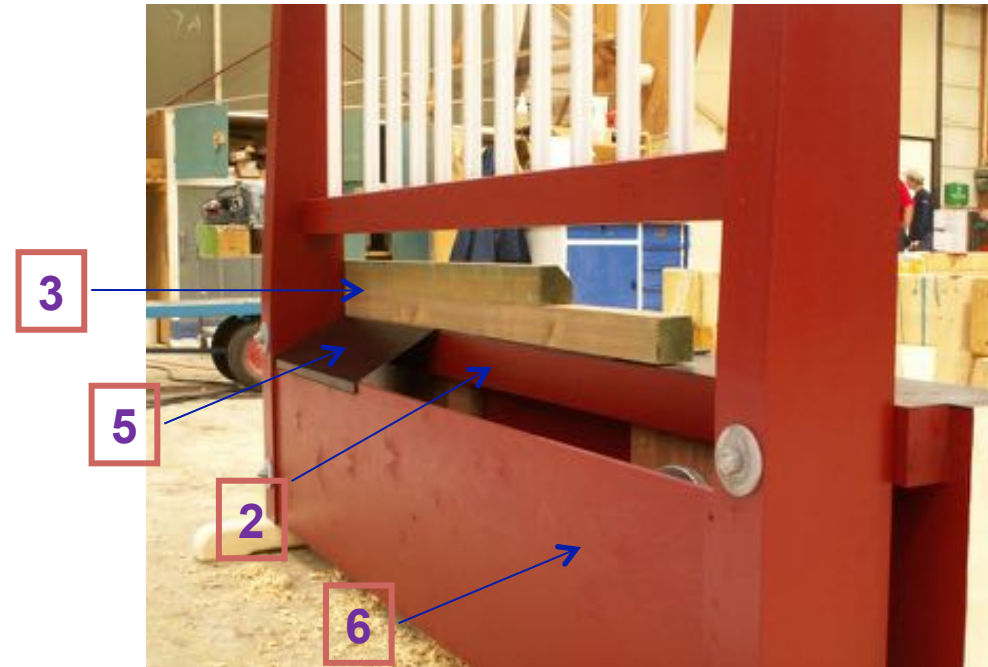
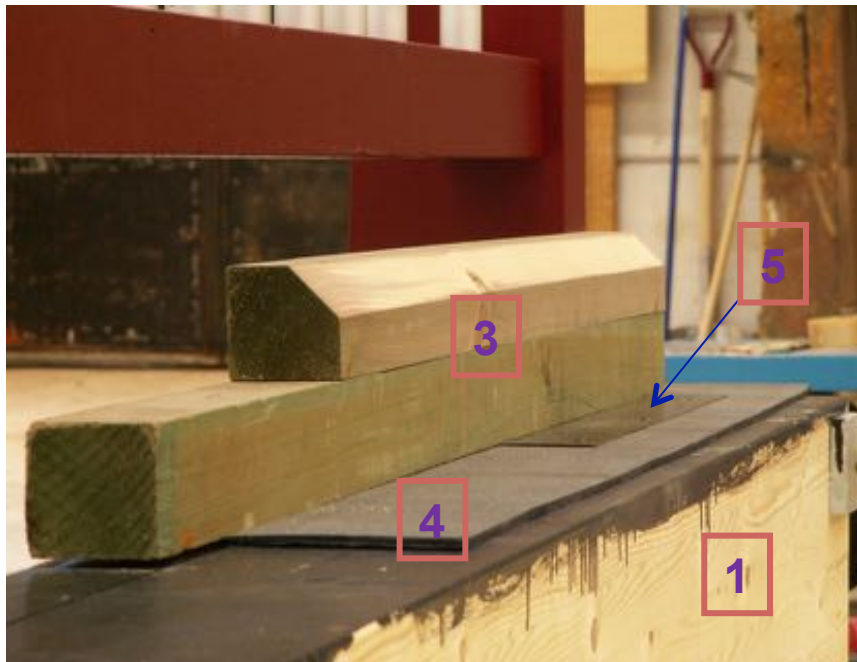


Sliding support



Fixed support

# Durability



- (1) Deck/beam
- (2) Wooden strip – it is screwed to the side of the beam/deck
- (3) Double wooden members – these are nailed to the wooden strip
- (4) Rubber mat 5mm,  $b=250$  mm
- (5) Steel plate  $t=0,6$  mm
- (6) Wearing planks/ wearing timber plate

# Durability



# Conclusions



- Glulam beams which are pre-stressed with steel bars, create very efficient stress-laminated timber decks
- The main advantage of SLT decks used as a bridge are:
  - Simplicity
  - Possibility to shape – also curved geometries
  - Durability
  - Sustainability of the materials
- The recognised negative aspects of SLT decks are:
  - effect of butt joints on the stiffness
  - loss of pre-stress after assembly



# Conclusions



- The design of SLT decks can be performed by e.g.
  - the theory of orthotropic plates using the finite element method,
  - simple calculation models, as shown in the Eurocode EN 1995-2
- Correct detailing is extremely important when it comes to deck durability